

TEOREMAS ENERGÉTICOS

① ENERGÍA ELÁSTICA DE DEFORMACIÓN EN PIEZAS PRISMÁTICAS.

A) Esfuerzo axial.

$$U_0 = \frac{N_x^2}{2EA^2} \Rightarrow U(N_x) = \frac{N_x^2 L}{2EA}$$

B) Momento flector

$$U_0 = \frac{M_z^2 y^2}{2EJ_z^2} \Rightarrow U(M_z) = \int_L \frac{M_z^2}{2EJ_z} dx$$

C) Esfuerzo cortante.

$$U_0 = \frac{V_y^2 Q_z^2}{2Gb^2 I_z^2} \Rightarrow U(V_y) = \int_L \frac{f_z V_y^2}{2GA} dx$$

D) Momento torsor

$$U_0 = \frac{M_x^2 r^2}{2GJ} \Rightarrow U(M_x) = \int_L \frac{M_x^2}{2GJ} dx$$

② TEOREMA DE CLAPEYRON

$$W = \frac{1}{2} \sum_{k=1}^n Q_k \Delta_k = U \quad (\text{comportamiento elástico lineal})$$

③ TEOREMAS DE RECIPROCIDAD

* Teorema de reciprocidad de Rayleigh y Betti.

$$\sum_{i=1}^n Q_i \cdot q_i^{(2)} = \sum_{j=1}^m P_j \cdot p_j^{(1)} \quad (\text{comportamiento elástico lineal})$$

* Teorema de reciprocidad de Maxwell.

$$q^{(2)} = p^{(1)} \quad (\text{comportamiento elástico lineal})$$

④ PRIMER TEOREMA DE CASTIGLIANO

$$Q_i = \frac{\partial U}{\partial \Delta_i} \quad (i=1, 2, \dots, n)$$

5) PRIMER TEOREMA DE ENGESSER Y SEGUNDO TEOREMA DE CASTIGLIANO.

* Primer teorema de Engesser

$$\Delta_i = \frac{\partial U^*}{\partial Q_i} \quad (i=1,2,\dots,n)$$

* Segundo teorema de Castigliano

$$\Delta_i = \frac{\partial U}{\partial Q_i} \quad (i=1,2,\dots,n) \quad (\text{comportamiento elástico lineal})$$

6) SEGUNDO TEOREMA DE ENGESSER. TEOREMA DEL TRABAJO MÍNIMO.
(Estructuras hiperestáticas)

$$\frac{\partial U^*}{\partial Q_i} = \Delta_i \quad (i=1,2,\dots,n)$$
$$\frac{\partial U^*}{\partial R_j} = 0 \quad (j=1,2,\dots,r)$$

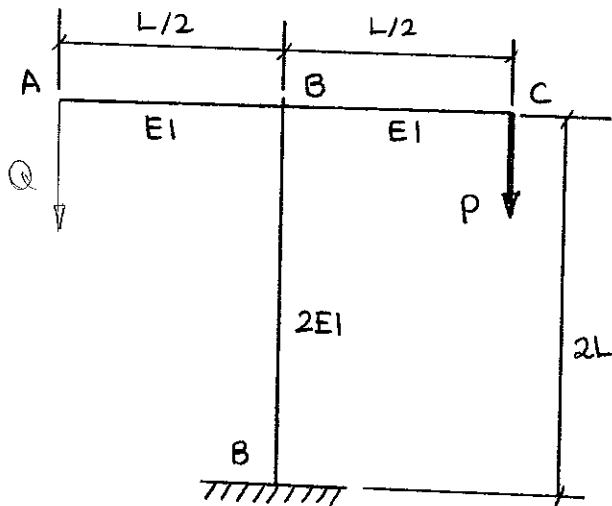
* Teorema de Castigliano del trabajo mínimo

$$\frac{\partial U}{\partial Q_i} = \Delta_i \quad (i=1,2,\dots,n)$$
$$\frac{\partial U}{\partial R_j} = 0 \quad (j=1,2,\dots,r)$$

(comportamiento elástico lineal)

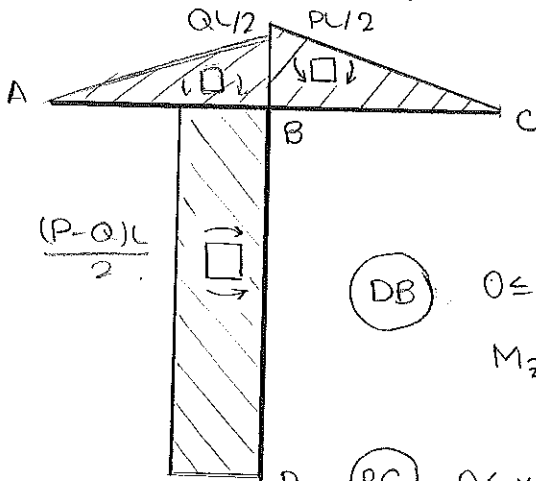
5. Teoremas energéticos

5.2



⊙ se desprecian los efectos debidos a los esfuerzos axiales y cortantes

DIAGRAMAS DE ESFUERZOS



SEGUNDO TEOREMA DE CASTIGLIANO.

$$\partial y_A = \frac{\partial U}{\partial Q} = \frac{1}{EI} \int M_2 \frac{\partial M_2}{\partial Q} dl; \quad \partial y_C = \frac{\partial U}{\partial P}$$

(DB) $0 \leq x \leq 2L$

$$M_2 = \frac{(P-Q)L}{2}; \quad \frac{\partial U}{\partial Q} = -\frac{L}{2}; \quad \frac{\partial U}{\partial P} = \frac{L}{2}$$

(BC) $0 \leq x \leq \frac{L}{2}$

$$M_2 = Px; \quad \frac{\partial U}{\partial Q} = 0; \quad \frac{\partial U}{\partial P} = x$$

(AB) $0 \leq x \leq \frac{L}{2}$

$$M_2 = Qx; \quad \frac{\partial U}{\partial Q} = x; \quad \frac{\partial U}{\partial P} = 0$$

$$\delta Y_A = \frac{1}{2EI} \int_0^{2L} \frac{(P-Q)L}{2} \cdot \frac{(L)}{2} dx + \frac{1}{EI} \int_0^{L/2} Qx^2 dx =$$

$$= \frac{1}{2EI} \cdot \frac{(Q-P)L^2}{4} (2L) + \frac{1}{EI} \cdot \frac{Q \cdot (L/2)^3}{3}$$

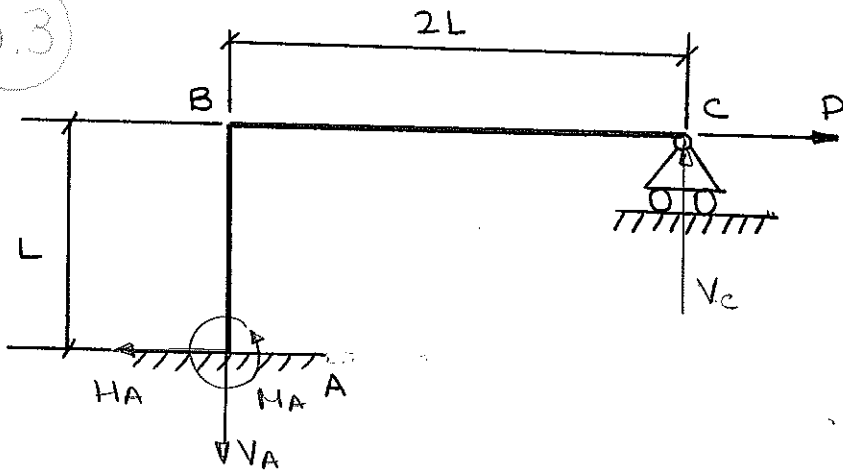
$$\delta Y_A = \{Q=0\} = -\frac{2PL^3}{8EI} = -\frac{PL^3}{4EI} \Rightarrow \boxed{\uparrow Y_A = \frac{PL^3}{4EI}}$$

$$\delta Y_C = \frac{1}{2EI} \int_0^{2L} \frac{(P-Q)L}{2} \cdot \frac{(L)}{2} dx + \frac{1}{EI} \int_0^{L/2} Px^2 dx =$$

$$= \frac{1}{2EI} \cdot \frac{(P-Q)L^2}{4} (2L) + \frac{1}{EI} \cdot \frac{P(L/2)^3}{3}$$

$$\delta Y_C = \{Q=0\} = \frac{7PL^3}{24EI} \Rightarrow \boxed{\downarrow Y_C = \frac{7PL^3}{24EI}}$$

5.3



- ⊗ Rigidez a flexión: EI
- ⊗ Se desprecian esfuerzos axiales y cortantes.

1) Hiperestático de grado 1. ($h=1$)



Ec. compatibilidad:
 $\delta y_c = 0$

SEGUNDO TEOREMA DE CASTIGLIANO.

$$\delta y_c = \frac{\partial U}{\partial Q} = \frac{1}{EI} \int M_z \frac{\partial M_z}{\partial Q} dl$$

(CB) ($0 \leq x \leq 2L$) $M_z = Q \cdot x$; $\frac{\partial M_z}{\partial Q} = x$

(BA) ($0 \leq x \leq L$) $M_z = P \cdot x - Q \cdot 2L$; $\frac{\partial M_z}{\partial Q} = -2L$

$$\delta y_c = \frac{\partial U}{\partial Q} = \frac{1}{EI} \left[\int_0^{2L} Qx^2 dx + \int_0^L (Px - 2QL)(-2L) dx \right] = \frac{1}{EI} \left[\frac{Q(2L)^3}{3} - PL^3 + 4QL^3 \right]$$

⊗ Ec. Compatibilidad: $\delta y_c = \frac{1}{EI} \left[\frac{8QL^3}{3} - PL^3 + 4QL^3 \right] = 0$

$$\frac{20}{3} Q = P \Rightarrow Q = \frac{3P}{20} = V_c$$

EQUILIBRIO.

$$V_A = V_c = \frac{3P}{20}$$

$$H_A = P$$

$$M_A + V_c \cdot 2L = P \cdot L \Rightarrow M_A = PL - \frac{6PL}{20} = \frac{7}{10} PL$$

⊗ Reacciones:

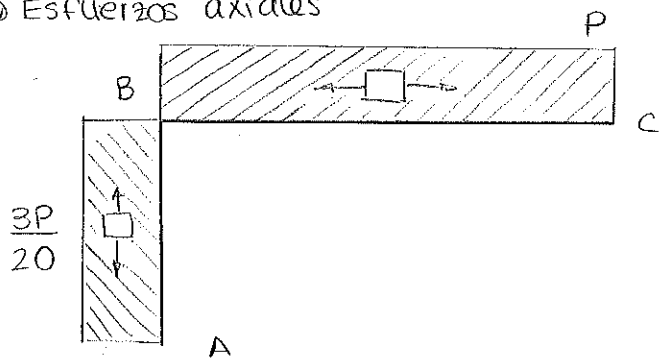
$$V_A = V_c = \frac{3P}{20}$$

$$H_A = P$$

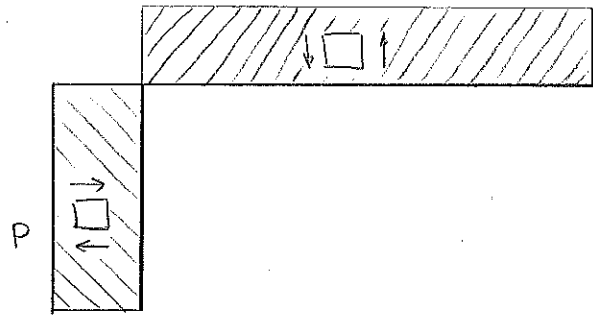
$$M_A = \frac{7}{10} PL$$

DIAGRAMAS DE ESFUERZOS

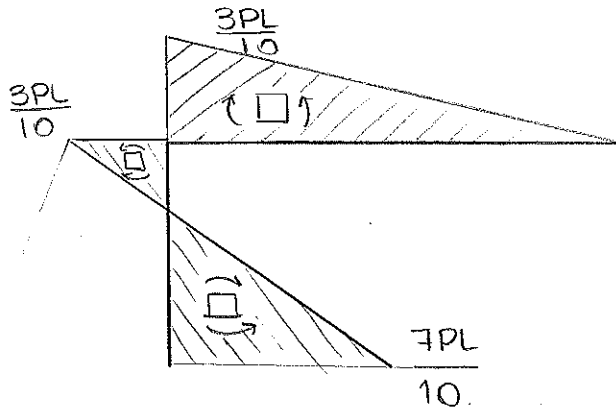
⊗ Esfuerzos axiales



⊗ Esfuerzos cortantes $\frac{3P}{20}$



⊗ Esfuerzos flectores.



2) SEGUNDO TEOREMA DE CASTIGLIANO

$$\vec{\delta}_{x_c} = \frac{\partial U}{\partial P}$$

$$\frac{\partial U}{\partial P} = \frac{1}{EI} \int M_2 \frac{\partial M_2}{\partial P} dl$$

$$\text{(CB)} \quad 0 \leq x \leq 2L \quad M_2 = Q \cdot x = \frac{3P}{20} x, \quad \frac{\partial M_2}{\partial P} = \frac{3x}{20}$$

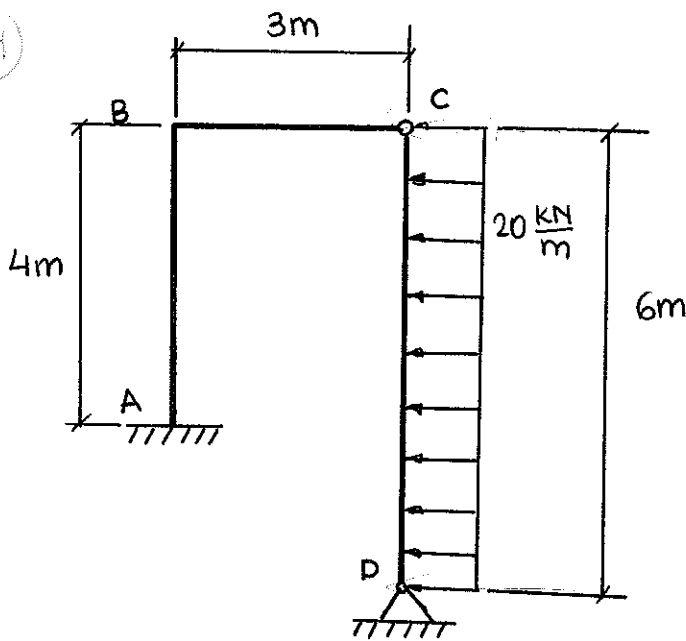
$$\text{(BA)} \quad 0 \leq x \leq L \quad M_2 = Px - Q \cdot 2L = Px - \frac{3P}{10} L; \quad \frac{\partial M_2}{\partial P} = x - \frac{3L}{10}$$

$$\frac{\partial U}{\partial P} = \frac{1}{EI} \left[\int_0^{2L} \frac{3P}{20} x \left(\frac{3x}{20} \right) dx + \int_0^L \left(Px - \frac{3P}{10} L \right) \left(x - \frac{3L}{10} \right) dx \right] =$$

$$= \frac{1}{EI} \left[\frac{9P}{400} \cdot \frac{8L^3}{3} + \frac{37PL^3}{300} \right] = \frac{11PL^3}{60}$$

$$\vec{\delta}_{x_c} = \frac{11PL^3}{60}$$

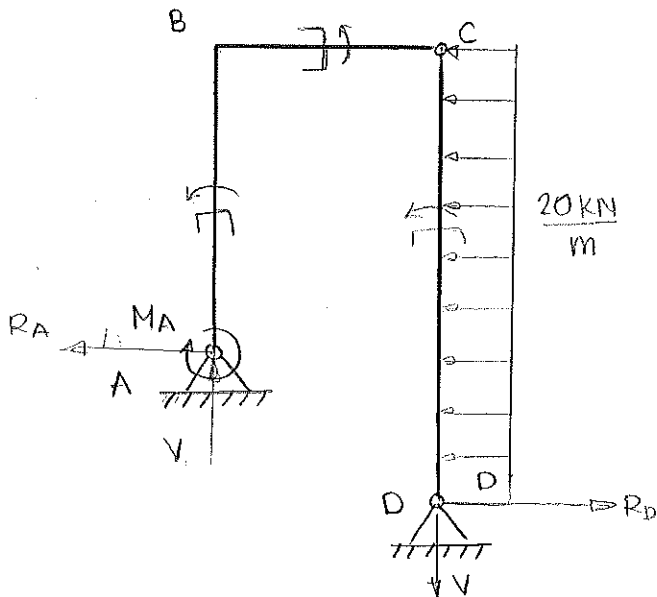
5.4



⊗ Rigidez a flexión: EI

⊗ Se desprecian los esfuerzos axiales y cortantes.

⊗ Estructura hiperestática de grado 1 $h=1 (M)$



E_c compatibilidad

$$\theta_A = 0$$

EQUILIBRIO

$$(1) M_A + V \cdot 3m + R_A \cdot 4m = 0 \Rightarrow V = \frac{240 \text{ kNm} - M_A}{3m}$$

$$(2) 20 \cdot 6 \text{ kN} \cdot 3m - R_D \cdot 6m = 0$$

$$R_D = 60 \text{ kN}$$

$$(3) R_D = R_A + 6 \cdot 20 \text{ kN} \Rightarrow R_A = -60 \text{ kN}$$

SEGUNDO TEOREMA DE CASTIGLIANO.

$$\theta_A = \frac{\delta U}{\delta M_A} = \frac{1}{EI} \int M_2 \frac{\partial M_2}{\partial M_A} dx$$

(AB) $0 \leq x \leq 4m$

$$M_2 = M_A - 60 \text{ kN} \cdot x ; \quad \frac{\partial M_2}{\partial M_A} = 1$$

$0 \leq x \leq 3m$

(BC) $M_2 = M_A - 60 \text{ kN} \cdot 4m + \frac{240 \text{ kNm} - M_A}{3} x ; \quad \frac{\partial M_2}{\partial M_A} = 1 - \frac{x}{3}$

(CD) $0 \leq x \leq 6m$

$$M_2 = -20 \text{ kN} \cdot x \cdot \frac{x}{2} + 60 \text{ kN} \cdot x ; \quad \frac{\partial M_2}{\partial M_A} = 0$$

$$\theta_A = \frac{\partial U}{\partial M_A} = \frac{1}{EI} \int_0^4 (M_A - 60x) dx + \frac{1}{EI} \int_0^3 \left[(M_A - 240) + \frac{240 - M_A}{3} x \right] \left(1 - \frac{x}{3} \right) dx = 0$$

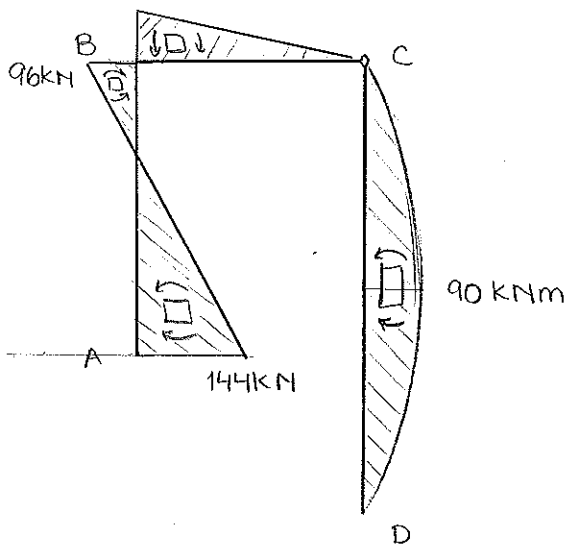
$$4M_A - \frac{4^2}{2} \cdot 60 + [M_A - 240] \int_0^3 \left(1 - \frac{x}{3} \right)^2 dx = 0$$

$$4M_A - 480 + (M_A - 240) \int_0^3 \left(1 - \frac{2x}{3} + \frac{x^2}{9} \right) dx = 0$$

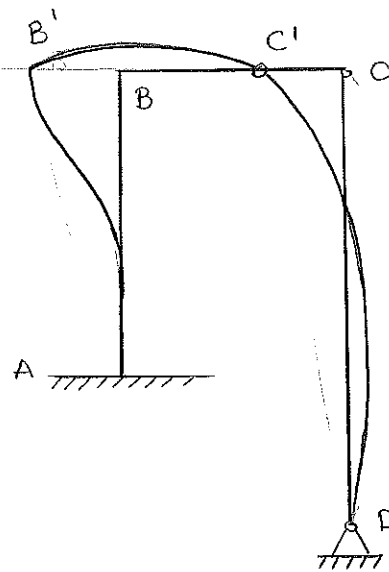
$$4M_A - 480 + (M_A - 240) \cdot \left(3 - \frac{2 \cdot 3^2}{3 \cdot 2} + \frac{3^3}{3 \cdot 9} \right) = 0$$

$$M_A = 144 \text{ kN}$$

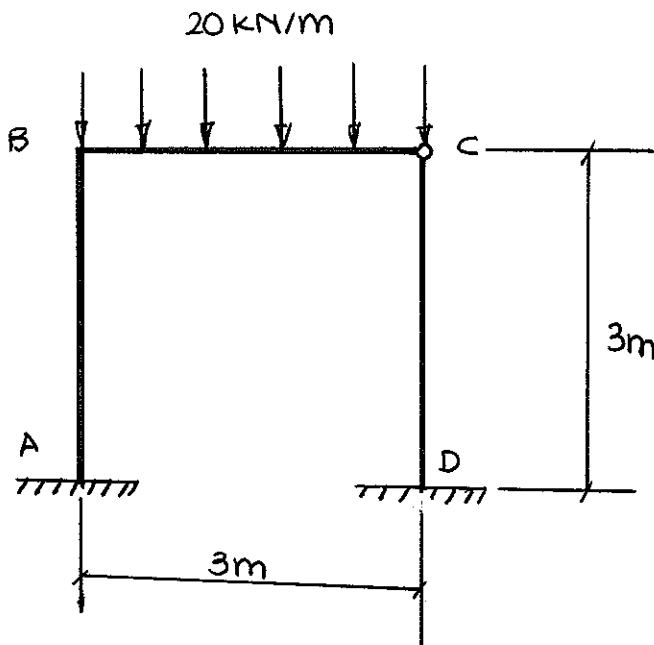
DIAGRAMA DE ESFUERZOS



DEFORMADA

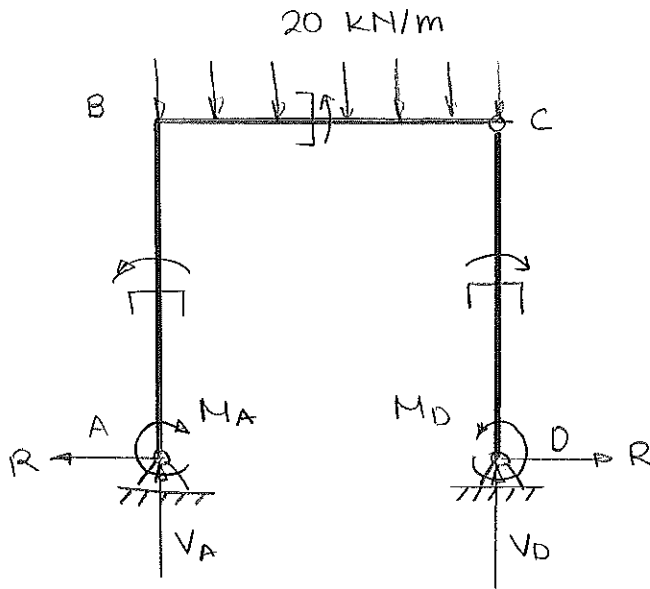


5.5



- ⊗ Rigidez EI
- ⊗ Se despreciará el efecto de esfuerzos cortantes y axiales.

⊗ ESTRUCTURA HIPERESTÁTICA: $n=2 (M_A, M_B)$



Ec. compatibilidad

$$\theta_A = 0$$

$$\theta_D = 0$$

EQUILIBRIO

(1) $V_A + V_D = 60 \text{ kN}$

(2) $M_D + 3R = 0 \Rightarrow R = -\frac{M_D}{3m}$

(3) $M_A + 3R + 3V_A - 60 \text{ kN} \cdot 1.5 \text{ m} = 0$

$$M_A + M_D + 3V_A - 90 \text{ kNm} = 0 \Rightarrow V_A = 30 \text{ kN} - \frac{M_A + M_D}{3m}$$

$$V_D = 30 \text{ kN} + \frac{M_A - M_D}{3m}$$

$$(AB) \quad 0 \leq x \leq 3m$$

$$M_2 = M_A + R_x = M_A - \frac{M_D}{3} x$$

$$\frac{\partial M_2}{\partial M_A} = 1; \quad \frac{\partial M_2}{\partial M_D} = -\frac{x}{3}$$

$$(BC) \quad 0 \leq x \leq 3$$

$$M_2 = M_A + 3R + V_A \cdot x - 20x \cdot \frac{x}{2} = M_A - M_D + 30x - \frac{M_A - M_D}{3} x - 10x^2$$

$$\frac{\partial M_2}{\partial M_A} = 1 - \frac{x}{3}; \quad \frac{\partial M_2}{\partial M_D} = -1 + \frac{x}{3}$$

$$(CD) \quad 0 \leq x \leq 3$$

$$M_2 = M_D + R \cdot x = M_D - \frac{M_D}{3} x$$

$$\frac{\partial M_2}{\partial M_A} = 0; \quad \frac{\partial M_2}{\partial M_D} = 1 - \frac{x}{3}$$

$$\theta_A = \frac{\partial U}{\partial M_A} = \frac{1}{EI} \int M_2 \frac{\partial M_2}{\partial M_A} dx = 0$$

$$\int_0^3 \left(M_A - \frac{M_D}{3} x \right) dx + \int_0^3 \left(M_A - M_D + 30x - \frac{M_A - M_D}{3} x - 10x^2 \right) \left(1 - \frac{x}{3} \right) dx = 0.$$

$$3M_A - \frac{M_D}{3} \cdot \frac{3^2}{2} + \int_0^3 \left(M_A - M_D + 30x - 2 \frac{M_A - M_D}{3} x - 20x^2 + \frac{10}{3} x^3 + \frac{M_A - M_D}{9} x^2 \right) dx$$

$$3M_A - \frac{3}{2} M_D + 3M_A + 3M_D + 30 \frac{3^2}{2} - 2 \frac{M_A - M_D}{3} \cdot \frac{3^2}{2} - 20 \frac{3^3}{3} + \frac{10}{3} \cdot \frac{3^4}{4} + \frac{M_A}{9} \frac{3^3}{3} - \frac{M_D}{9} \frac{3^3}{3}$$

$$\boxed{4M_A - \frac{5}{2} M_D = -45/2}$$

$$\theta_B = \frac{\partial U}{\partial M_B} = \frac{1}{EI} \int M_2 \frac{\partial M_2}{\partial M_B} dx = 0.$$

$$\int_0^3 \left(M_A - \frac{M_D}{3} x \right) \left(-\frac{x}{3} \right) dx + \int_0^3 \left(M_A - M_D + 30x - \frac{M_A - M_D}{3} x - 10x^2 \right) \left(1 + \frac{x}{3} \right) dx + \int_0^3 M_D \left(1 - \frac{x}{3} \right)^2 dx = 0.$$

$$\int_0^3 \left(M_A \frac{x}{3} + M_D \frac{x^2}{9} \right) dx - 3M_A + 3M_D + 30 \frac{3^2}{2} + 2 \frac{M_A - M_D}{3} \cdot \frac{3^2}{2} + 20 \frac{3^3}{3} - \frac{10}{3} \cdot \frac{3^4}{4} - M_A + M_D$$

$$+ M_D \int_0^3 \left(1 - \frac{2x}{3} + \frac{x^2}{9} \right) dx = 0$$

$$-M_A \cdot \frac{3^2}{2 \cdot 3} + \frac{M_D}{9} \cdot \frac{3^3}{3} - 3M_A + 3M_D - 30 \cdot \frac{3^2}{2} + 2 \frac{M_A + M_D}{3} \cdot \frac{3^2}{2} + 20 \frac{3^3}{3} - \frac{10}{3} \cdot \frac{3^4}{4} - M_A + M_D$$

$$+ M_D \cdot 3 - M_D \cdot \frac{2}{3} \cdot \frac{3^2}{2} + M_D \cdot \frac{3^3}{9 \cdot 3} = 0.$$

$$-\frac{5}{2}M_A + 3M_D = +45/2$$

$$M_A = -1'9565 \text{ KNm}$$

$$M_D = +5'8696 \text{ KNm}$$

$$R = R_A = R_B = -1'9565 \text{ KN}$$

$$V_A = 32,6087 \text{ KN}$$

$$V_D = 27,3913 \text{ KN}$$



5.6

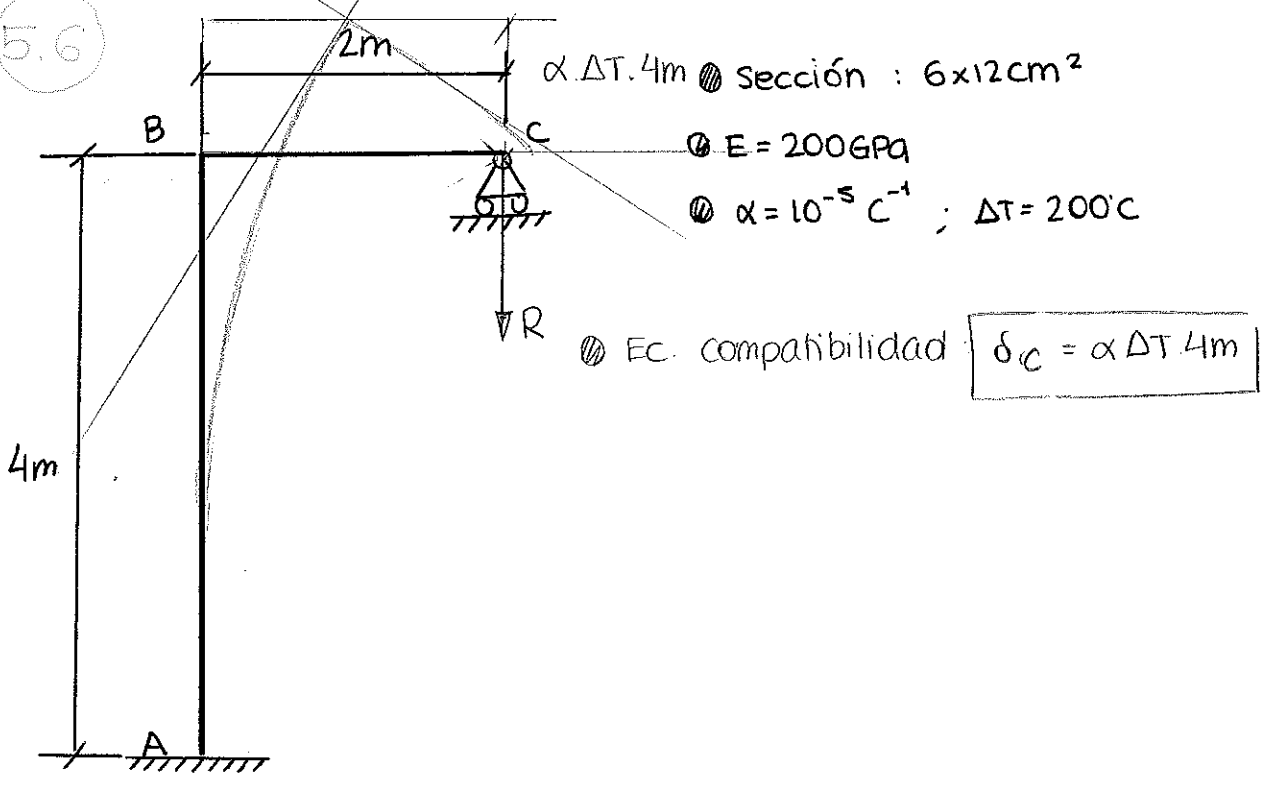
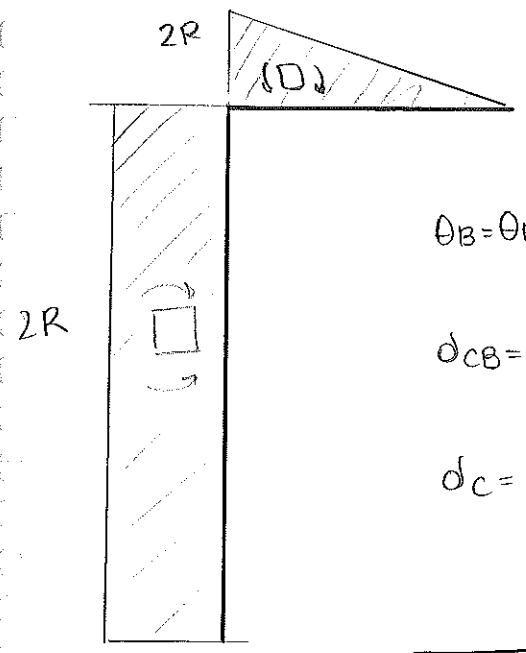


DIAGRAMA DE ESFUERZOS FLECTORES.



$$\delta_C = \theta_B \cdot 2m + d_{CB}$$

$$\theta_B = \theta_{BA} = \frac{1}{EI} [2R \cdot 4] = \frac{8R}{EI}$$

$$d_{CB} = \frac{1}{EI} \left[\frac{1}{2} 2R \cdot 2 \cdot \frac{2}{3} \cdot 2 \right] = \frac{8R}{3EI}$$

$$\delta_C = \frac{16R}{EI} + \frac{8R}{3EI} = 4\alpha \Delta T$$

$$R = 740,571 \text{ N}$$

$$\theta_B = 3,429 \cdot 10^{-3} \text{ rad}$$

Por energías...

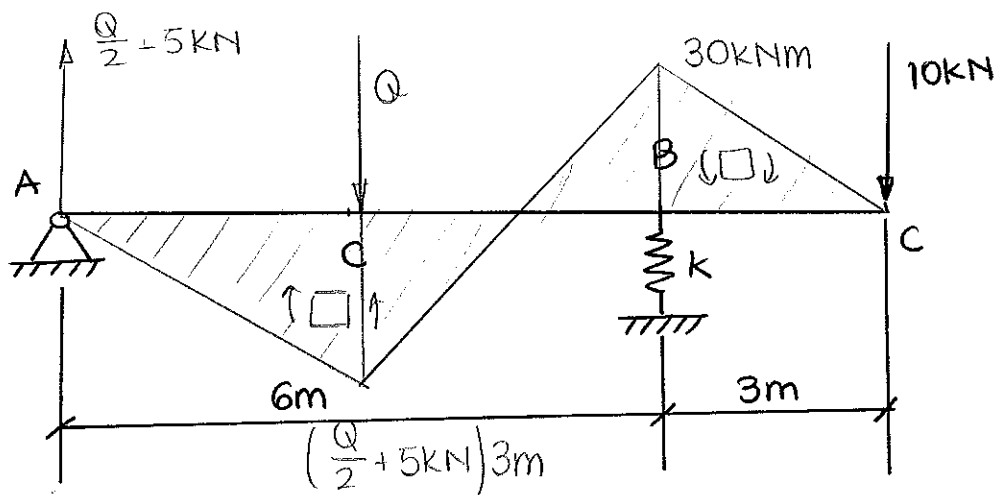
$$\delta_C = \frac{\partial U}{\partial R}$$

$$U = \int_0^4 \frac{(2R)^2}{2EI} dx + \int_0^2 \frac{(R \cdot x)^2}{2EI} dx = \frac{7R^2}{1296000}$$

$$\delta_C = \frac{7R}{648000} = \alpha \Delta T \cdot 4m \Rightarrow R = 740,571 \text{ N}$$



5.7



$$\bullet E = 200 \text{ GPa}$$

$$\bullet I = 1450 \text{ cm}^4$$

$$\boxed{AC} \quad 0 \leq x \leq 3\text{m}$$

$$M_z = \left(\frac{Q}{2} - 5\text{kN}\right)x \quad \frac{\partial M_z}{\partial Q} = \frac{x}{2}$$

$$\boxed{CB} \quad 3\text{m} \leq x \leq 6\text{m}$$

$$M_z = \left(\frac{Q}{2} - 5\text{kN}\right)x - Q(x - 3\text{m}) = -\frac{Q}{2}x + 3\text{m} \cdot Q + 5\text{kN} \cdot x$$

$$\frac{\partial M_z}{\partial Q} = -\frac{x}{2} + 3$$

$$\boxed{BC} \quad 0 \leq x \leq 3$$

$$M_z = 10\text{k} \cdot x$$

$$\frac{\partial M_z}{\partial Q} = 0$$

$$\Delta_c = \frac{\partial U}{\partial Q} = \frac{1}{EI} \int_0^3 \left(\frac{Q}{2} - 5\text{kN}\right) \frac{x^2}{2} dx + \frac{1}{EI} \int_3^6 \left(-\frac{Q}{2}x + 3Q - 5x\right) \left(3 - \frac{x}{2}\right) dx + \frac{1}{2k} = 0$$

$$U_m = \frac{1}{2} \frac{R^2}{k} = \frac{1}{2} \frac{\left(\frac{Q}{2} + 15\text{kN}\right)^2}{k} = \frac{1}{2k} \left(\frac{Q^2}{4} + 15Q + 15^2\right)$$

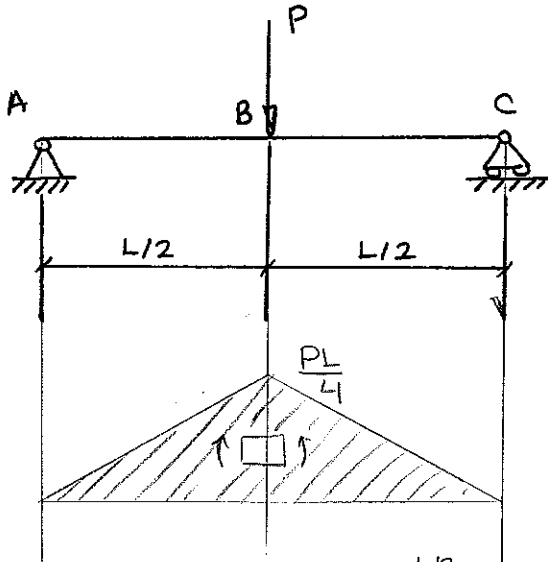
$$\frac{\partial U_m}{\partial Q} = \frac{+15}{2k} + \frac{Q}{4k}$$

$$\boxed{k = 322,22 \text{ kN/m}}$$

(...) Ponemos como incógnita 10kN y lo mismo.



C-5.1



- Rigidez: EI
- Se desprecia el efecto del esfuerzo cortante.

$$M_{z1} = \frac{P}{2}x$$

$$M_{z2} = -\frac{P}{2}x$$

A)

$$U = \int_0^{L/2} \frac{M_{z1}^2}{2EIz} dz + \int_0^{L/2} \frac{M_{z2}^2}{2EIz} dz = \int_0^{L/2} \frac{(P/2)x^2}{2EIz} dz + \int_0^{L/2} \frac{(P/2)x^2}{2EIz} dz =$$

$$= 2 \cdot \int_0^{L/2} \frac{P^2}{4EI \cdot 2} x^2 dz = \frac{P^2}{4EI} \frac{(L/2)^3}{3} = \frac{P^2 L^3}{96EI}$$

$$U = \frac{P^2 L^3}{96EI}$$

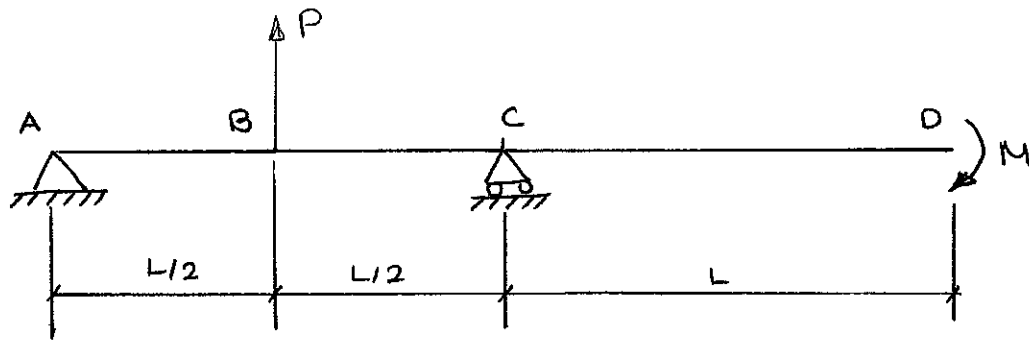
B) $W = U = \frac{1}{2} P \cdot \delta_c$

$$\delta_c = \delta_{CB} = \frac{1}{EI} \left[\frac{1}{2} \frac{P}{4} \cdot \frac{L}{2} \cdot \frac{2}{3} \frac{L}{2} \right] = \frac{PL^3}{48EI}$$

$$U = \frac{P^2 L^3}{96EI}$$

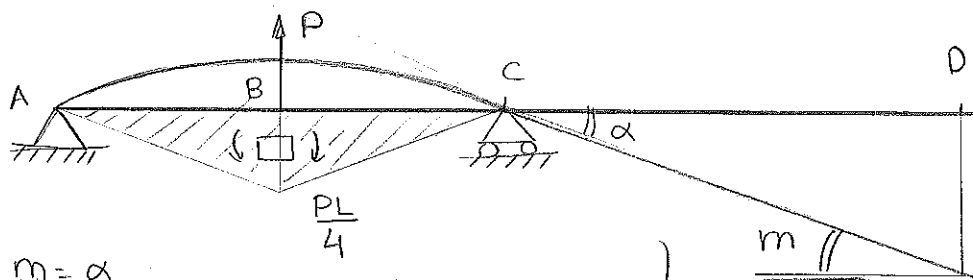


C-5.2



TEOREMA DE RECIPROCIDAD DE RAYLEIGH Y BETTI

$$\sum Q_i \cdot q_i^{(2)} = \sum P_j \cdot p_j^{(1)}$$



$$m = \alpha$$

$$\alpha = \frac{\delta_{AC}}{L}$$

$$\delta_{AC} = \frac{1}{EI} \left(\frac{1}{2} \cdot \frac{PL}{4} \cdot \frac{L}{2} \cdot L \right) = \frac{PL^3}{16EI}$$

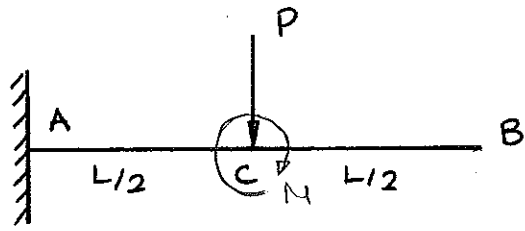
$$m = \frac{PL^2}{16EI}$$

$$M \cdot m = P \cdot p \Rightarrow p = \frac{M \cdot \frac{PL^2}{16EI}}{P} = \frac{ML^2}{16EI}$$

$$y_B = p = \frac{ML^2}{16EI}$$



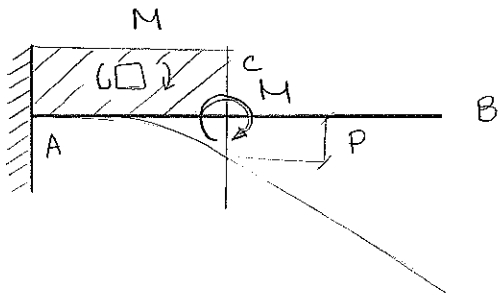
C-5.4



● Rigidez a flexión = EI

TEOREMA DE RECIPROCIDAD DE RAYLEIGH Y BETI

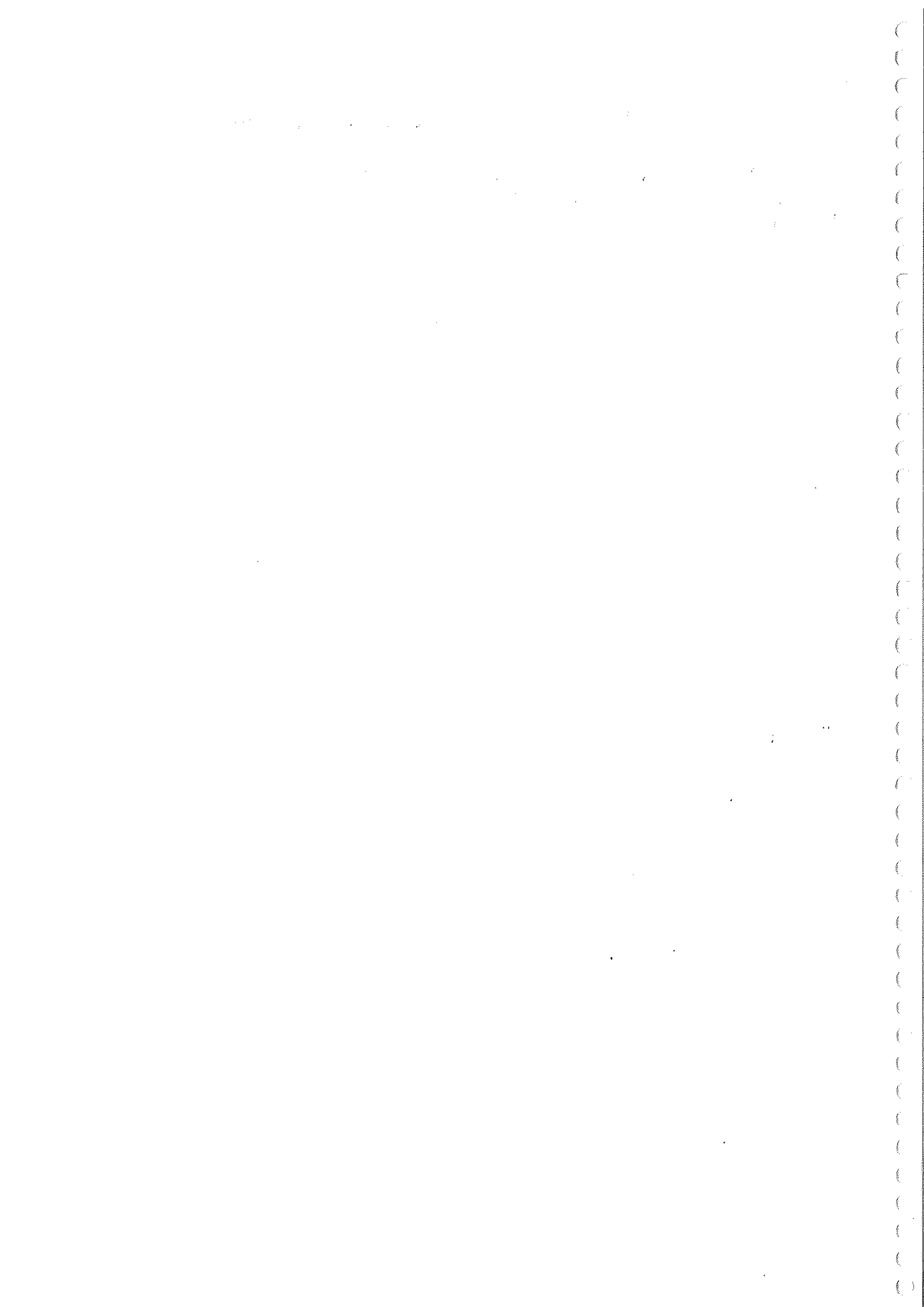
$M \cdot m = P \cdot p$



$$P = \delta_{CA} = \frac{1}{EI} \left[M \cdot \frac{L}{2} \cdot \frac{L}{4} \right] = \frac{ML^2}{8EI}$$

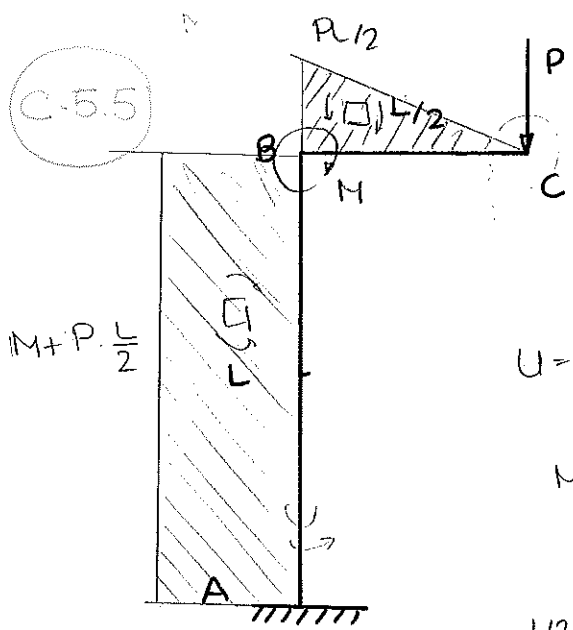
$$M \cdot m = P \cdot p \Rightarrow m = \frac{P \cdot p}{M} = \frac{PL^2}{8EI}$$

$$\theta_C = m = \frac{PL^2}{8EI}$$



C-5.5

SEGUNDO TEOREMA DE CASTIGLIANO.



$$\Delta i = \frac{\partial U}{\partial Q_i}$$

$$U = \int_0^L \frac{M_1^2}{2EI} dx + \int_0^{L/2} \frac{M_2^2}{2EI} dx$$

$$M_{21} = M + \frac{PL}{2}; \quad M_{22} = P \cdot x$$

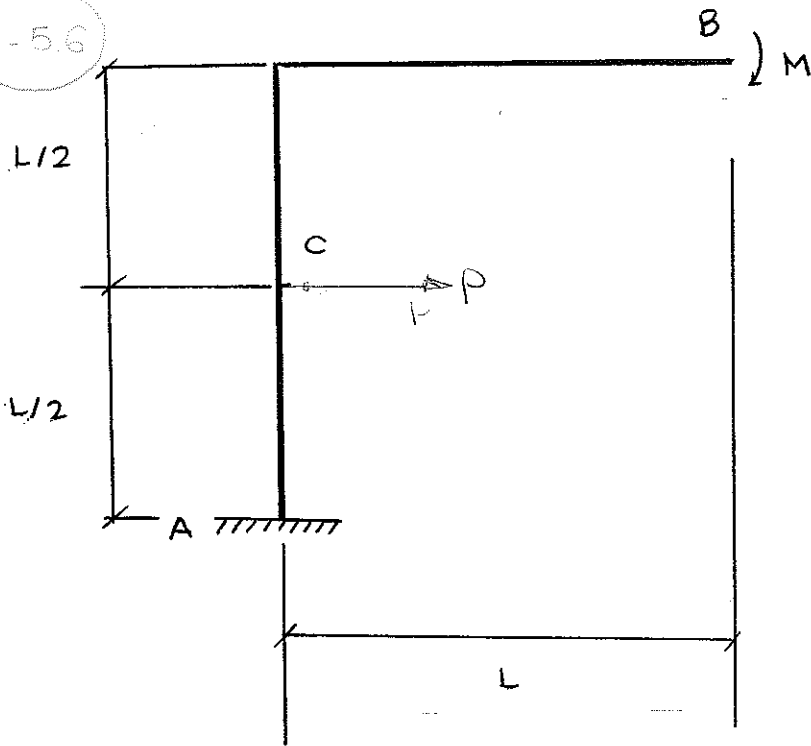
$$U = \int_0^L \frac{(M + PL/2)^2}{2EI} dx + \int_0^{L/2} \frac{(Px)^2}{2EI} dx = \frac{M^2 + MPL + (PL/2)^2}{2EI} \cdot L + \frac{P^2 \cdot (L/2)^3}{6EI}$$

$$\theta_B = \frac{\partial U}{\partial M} = \frac{(2M + PL)L}{2EI}$$

$$M = 0 \Rightarrow \theta_B = \frac{PL^2}{2EI}$$

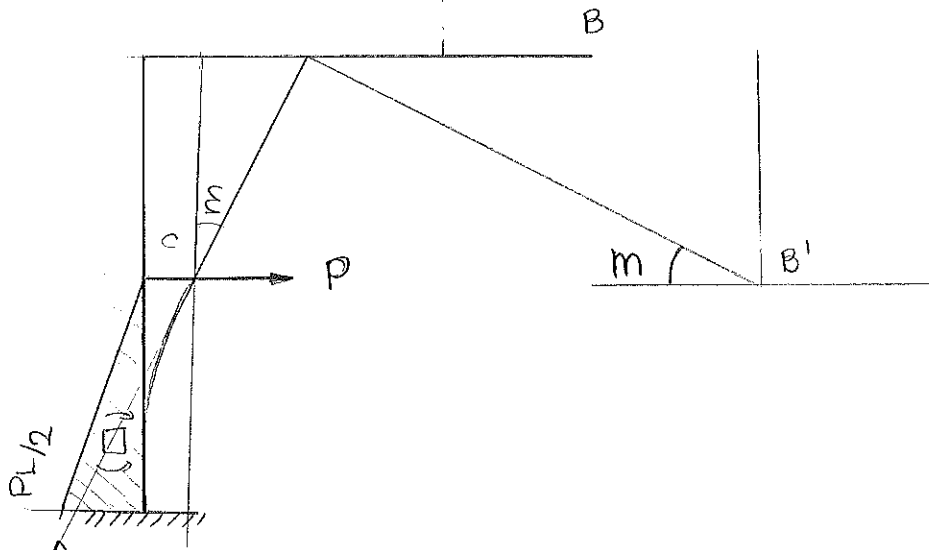


C-5.6



● Rigidez a flexión: EI

TEOREMA DE RECIPROCIDAD DE RAYLEIGH Y BETTI

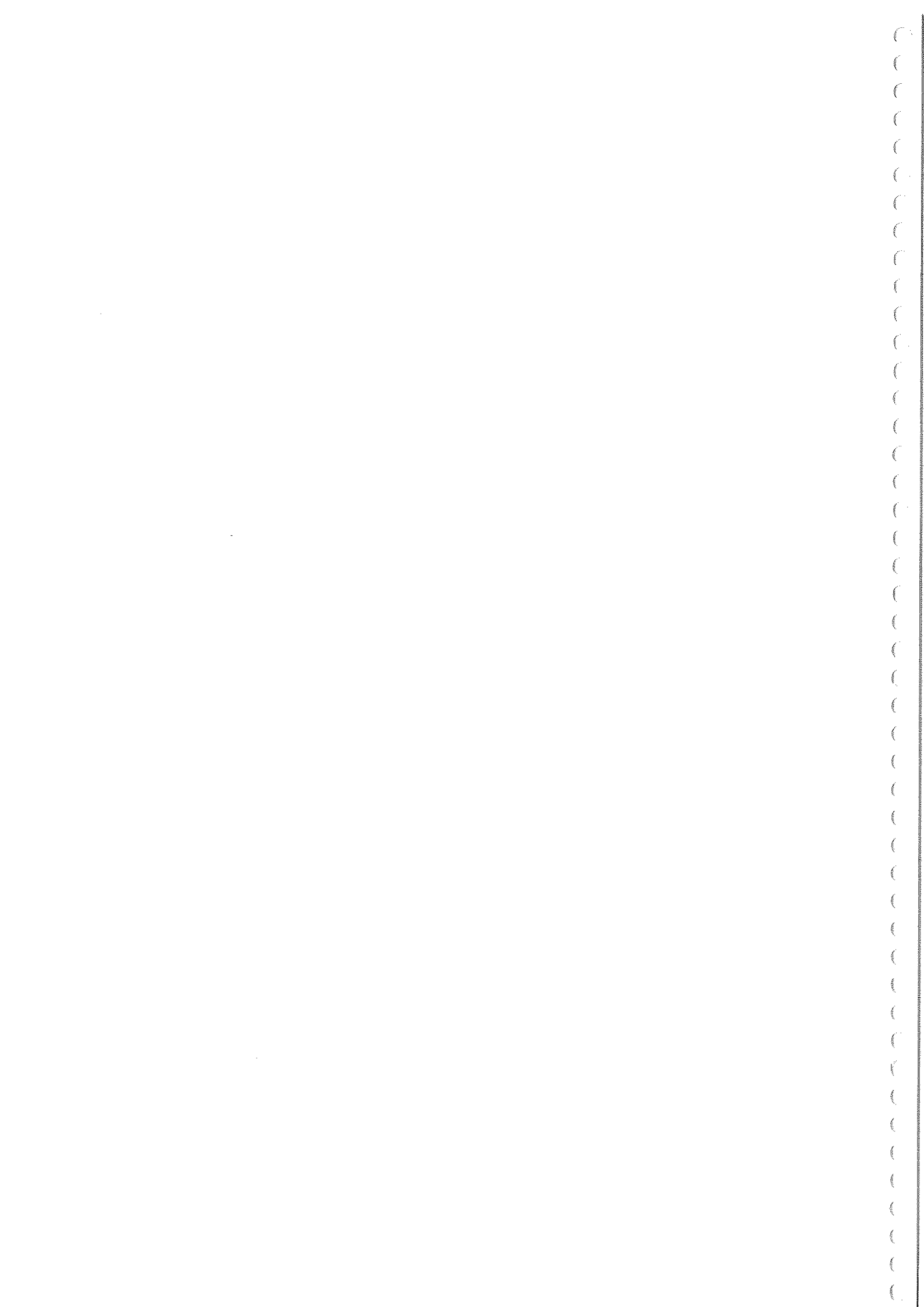


$$m = \theta_{CA} = \frac{1}{EI} \cdot \frac{1}{2} \cdot \frac{PL}{2} \cdot \frac{L}{2} = \frac{PL^2}{8EI}$$

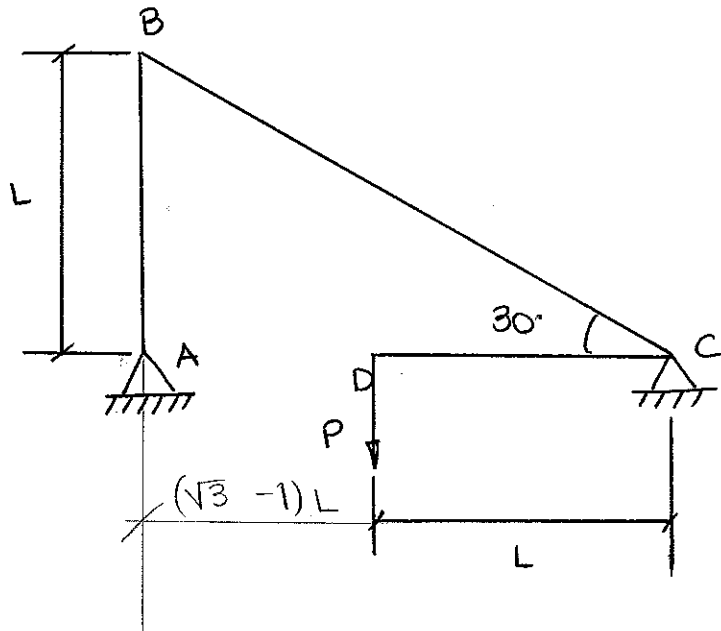
$$P \cdot p = M \cdot m$$

$$p = \frac{M \cdot PL^2 / (8EI)}{P} = \frac{ML^2}{8EI}$$

$$\delta_c = p = \frac{ML^2}{8EI}$$



C-5.A



Se despreciará el efecto de los esfuerzos axial y cortante en el cálculo de las tensiones y deformaciones.

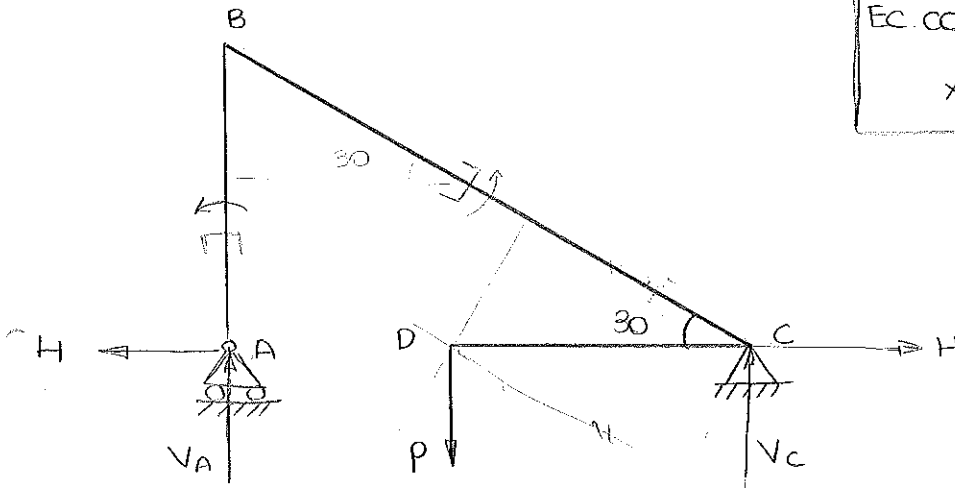
$P = 32,4 \text{ kN}$

$S = 10 \times 12 \text{ cm}^2$

$L = 100 \text{ cm}$

$E = 200 \text{ GPa}, \sigma_f = 270 \text{ MPa}$

ESTRUCTURA HIPERESTÁTICA : $n=1$ (H)



Ec. compatibilidad:

$\chi_A = 0$

EQUILIBRIO

$M_A = 0: P(\sqrt{3}-1)L - V_C \sqrt{3}L = 0 \Rightarrow V_C = \frac{P(\sqrt{3}-1)}{\sqrt{3}}$

$\Sigma F_y = 0: V_A + V_C = P \Rightarrow V_A = P + \frac{1}{\sqrt{3}}P - P = \frac{P}{\sqrt{3}} \Rightarrow V_A = \frac{P}{\sqrt{3}}$

(AB) $0 < x < L$

$M_2 = Hx, \quad \frac{\partial M_2}{\partial H} = x$

(BC) $0 < x < 2L$

$M_2 = V_A \cdot x - \frac{\sqrt{3}}{2} + H \cdot (L - \frac{x}{2})$

(CD) $0 < x < L$

$M_2 = Px; \quad \frac{\partial M_2}{\partial H} = 0$

$\frac{\partial M_2}{\partial H} = (L - \frac{x}{2})$

$$x_A = \frac{\partial U}{\partial H} = \int M_2 \frac{\partial M_2}{\partial H} d\ell = 0$$

$$\frac{1}{EI} \int_0^L Hx^2 dx + \frac{1}{EI} \int_0^{2L} \left[\frac{Px}{2} + H \left(L - \frac{x}{2} \right) \right] \left(L - \frac{x}{2} \right) dx = 0.$$

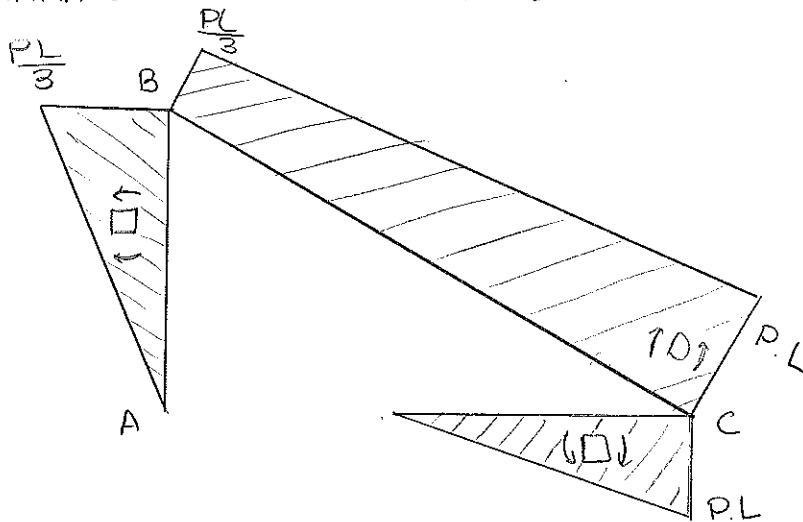
$$H \cdot \frac{L^3}{3} + \int_0^{2L} \frac{P}{2} \left(xL - \frac{x^2}{2} \right) dx + \int_0^{2L} H \left(L - \frac{x}{2} \right)^2 dx =$$

$$= H \cdot \frac{L^3}{3} + \frac{P}{2} \left[L \cdot \frac{(2L)^2}{2} - \frac{(2L)^3}{2 \cdot 3} \right] + HL^2(2L) - HL \frac{(2L)^2}{2} + \frac{H}{4} \cdot \frac{(2L)^3}{3} = 0.$$

$$H \left[\frac{L^3}{3} + 2L^3 - \frac{4L^3}{2} + \frac{8L^3}{12} \right] + \frac{P}{2} \left[\frac{4L^2}{2} - \frac{8L^3}{6} \right] = 0.$$

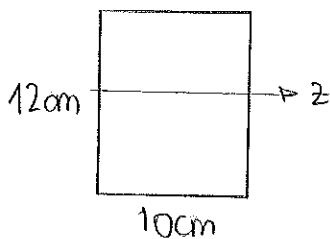
$$H = -\frac{1}{3}P$$

DIAGRAMA DE MOMENTOS FLECTORES.



ESFUERZO máximo : C.

$$M_2 = P \cdot L$$



$$I_2 = \frac{1}{12} 100 \text{ cm} (12 \text{ cm})^3 = 1.44 \cdot 10^{-5} \text{ m}^4$$

$$\sigma_{xx} (\text{máx}) = \frac{P \cdot L \cdot 6 \text{ cm}}{I_2} = 135 \text{ MPa}$$

$$n = \frac{\sigma_f}{\sigma_{xx, \text{max}}} = 2$$

$$n = 2$$

SEGUNDO TEOREMA DE CASTIGLIANO

$$\delta_D = \frac{\delta U}{\delta P} = \frac{1}{EI} \int M_2 \frac{\partial M_2}{\partial P} dx$$

$$\textcircled{AB} \quad 0 < x < L$$

$$M_2 = H \cdot x = -\frac{P}{3}x; \quad \frac{\partial M_2}{\partial P} = -\frac{x}{3}$$

$$\textcircled{BC} \quad 0 < x < 2L$$

$$M_2 = \frac{Px}{2} + \frac{P}{3}\left(L - \frac{x}{2}\right) = -\frac{PL}{3} + 2\frac{Px}{3}$$

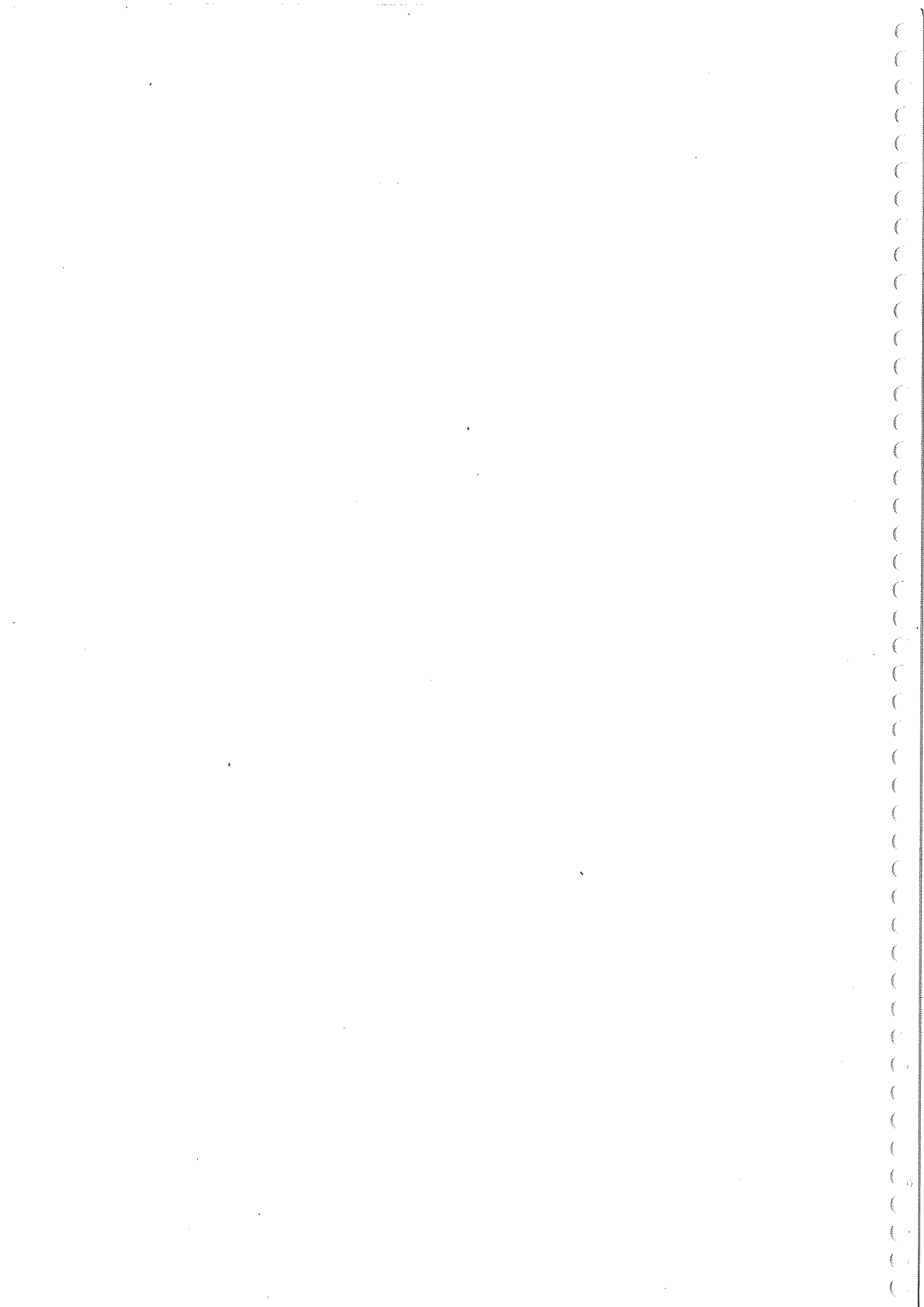
$$\frac{\partial M_2}{\partial P} = \frac{2x+L}{3}$$

$$\textcircled{CD} \quad 0 < x < L$$

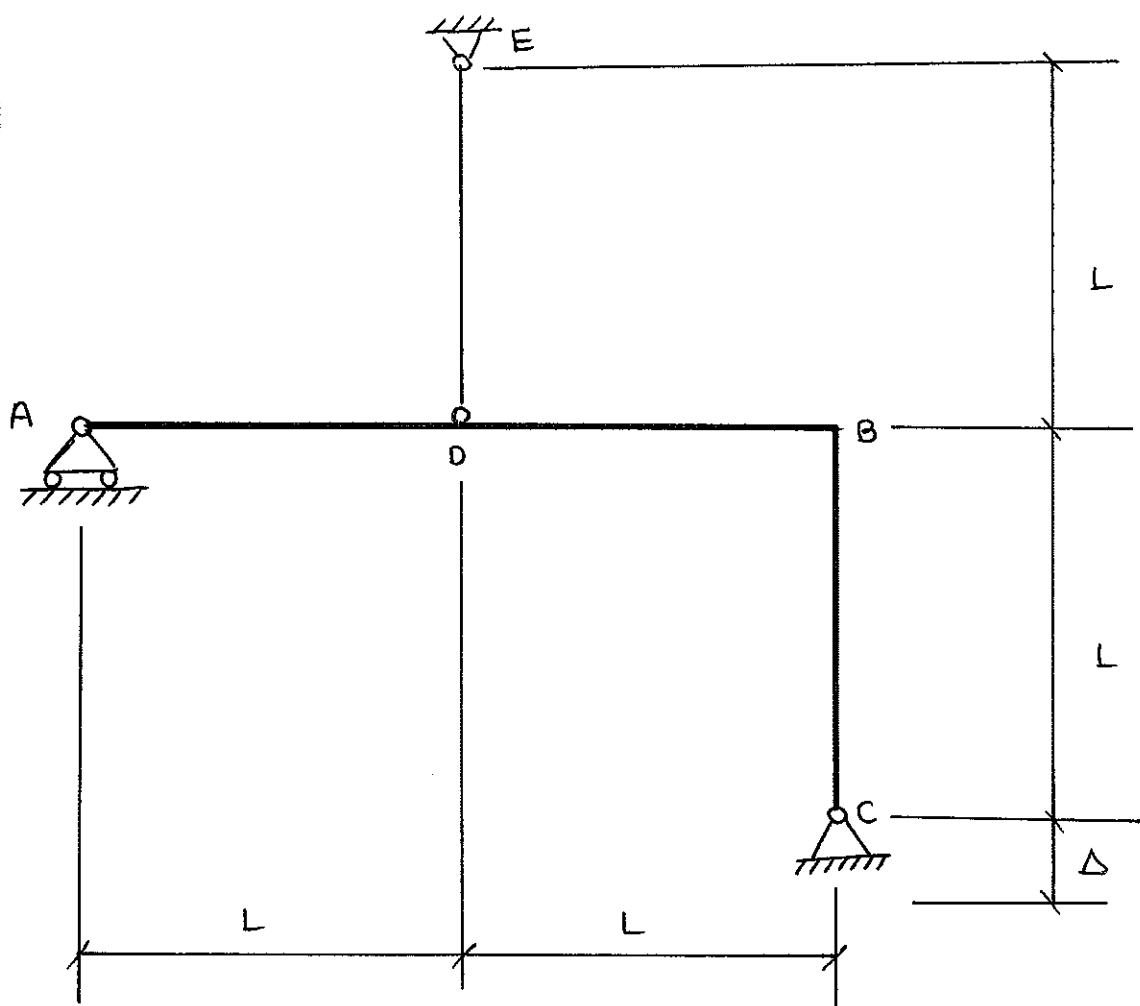
$$M_2 = P \cdot x; \quad \frac{\partial M_2}{\partial P} = x$$

$$\delta_D = \frac{1}{EI} \left[\int_0^L \frac{P}{3}x \cdot \frac{x}{3} dx + \int_0^{2L} \left(-\frac{PL}{3} + 2\frac{Px}{3}\right) \frac{2x+L}{3} dx + \int_0^L Px^2 dx \right] = 0,01 \text{ m}$$

$$\delta_D = 1 \text{ cm}$$



C-58



● Barras: $S = 8 \times 12 \text{ cm}^2$

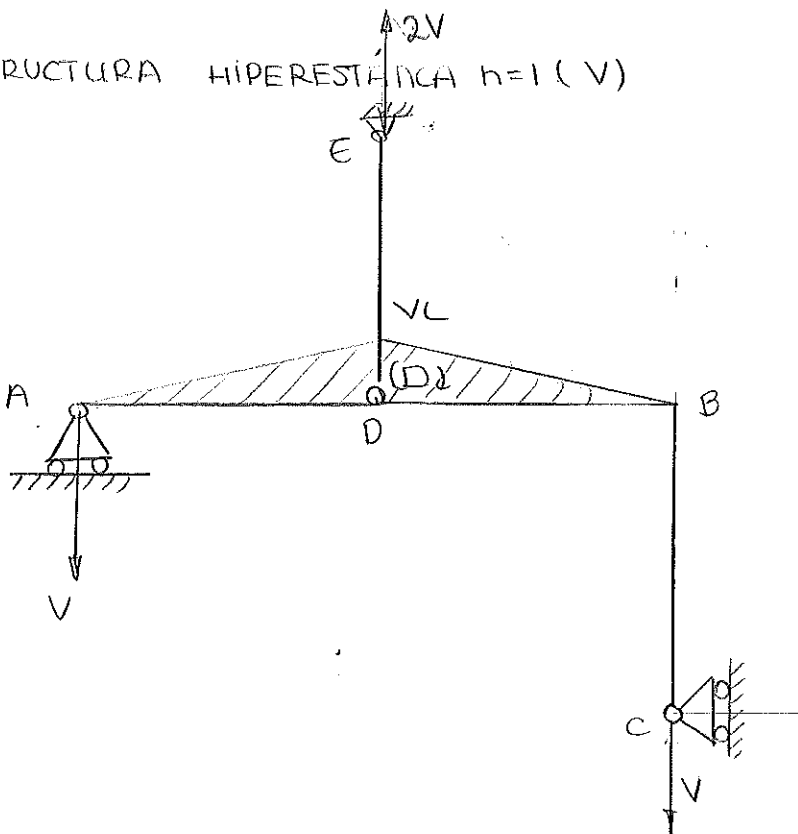
● Tirante: $S = 2 \text{ cm} \times 2 \text{ cm}$

● $L = 1 \text{ m}$

● $E = 200 \text{ GPa}$; $\sigma_f = 280 \text{ MPa}$.

ESTRUCTURA HIPERESTÁTICA $n=1$ (V)

Ec. compatibilidad:
 $\delta_c = \Delta$



$$U = 2 \int_0^L \frac{M_2^2}{2EI} dx + \frac{N_x^2 L}{2EA_T} = 2 \int_0^L \frac{V^2 x^2}{2EI} dx + \frac{4V^2 L}{2EA_T} =$$

$$= \frac{V^2}{EI} \cdot \frac{L^3}{3} + \frac{2V^2 L}{EA_T}$$

$$\delta_c = \frac{\partial U}{\partial V} = \frac{2VL^3}{3EI} + \frac{4VL}{EA_T} = \Delta$$

$$V \left(\frac{2L^3}{3EI} + \frac{4L}{EA_T} \right) = \Delta \Rightarrow V = \frac{\Delta}{\left(\frac{2L^3}{3EI} + \frac{4L}{EA_T} \right)} = 2946793,997 \Delta \text{ (N)}$$

$$A_T = (2\text{cm})^2 = 4 \cdot 10^{-4} \text{ m}^2$$

$$I = \frac{1}{12} 8\text{cm}(12\text{cm})^3 = 1,152 \cdot 10^{-5}$$

$$N_x = 2V = 5893587,995 \Delta \text{ (N)} \Rightarrow \boxed{N = 5893'588 \Delta \text{ (kN)}}$$

$$\sigma_{xx} = \frac{N_x}{A_T} = 147,3397 \Delta \text{ (MPa)}$$

ESFUERZO AXIAL

$$\sigma_{xx} = 147,3397 \Delta \quad \frac{\sigma_f}{\sigma_{xx}} = n \Rightarrow \frac{\sigma_f}{147,3397 \Delta} = 1,3 \Rightarrow \Delta_{\text{máx}} = 1'4618 \text{ m}$$

ESFUERZO FLECTOR

$$M_2 = V \cdot L = 2946793,997 \Delta \text{ (Nm)}$$

$$\sigma_{xx} = \frac{M_2 \cdot y}{I_2} = 1'53478854 \cdot 10^{10} \Delta \text{ (Pa)} = 15347,8854 \Delta \text{ (MPa)}$$

$$\frac{\sigma_f}{\sigma_{xx}} = 1,3 \Rightarrow \Delta_{\text{máx}} = 0'014 \text{ m} = 14 \text{ mm}$$

$$\boxed{\Delta_{\text{máx}} = 14 \text{ mm Fallo en D por flexión}}$$